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Soil Conservation Service

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FLOOD PLAIN MANAGEMENT

A Study Of South Fork Shenandoah Tributaries

Rockingham County, Virginia

APPENDIX I STONY RUN

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FOREWORD

The main report on the Flood Plain Management Study of South Fork Shenandoah River Tributaries provides information and data needed for use by administrators and the general public. Discussion of findings and recommendations relevant to the total study area are included.

Eight appendixes or technical reports include specifics on each tributary as listed below. Tables, flood profiles and area-flooded photomaps provide information for user agencies and individuals to make technical decisions and to comply with regulations related to the use of flood plains.

Appendix I Stony Run

Appendix II Quail Run - Boone Run Appendix III Cub Run - Big Run

Appendix IV Naked Creek

Appendix V Dry Run

Appendix VI Hawksbill Creek

Appendix VII Mill Creek - Congers Creek

Appendix VIII Pleasant Run

We thank those who contributed their active interest, cooperation, and information to this project.



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APPENDIX I South Fork Shenandoah River Tributaries FLOOD PLAIN MANAGEMENT STUDY Technical Report STONY RUN Rockingham County, Virginia

INTRODUCTION

This technical report on Stony Run is one of eight such appendixes to the Flood Plain Management Study on South Fork Shenandoah River Tributaries. The main report includes items such as authorities, responsibilities, scope, procedures, description, recommendations, and data common to the tributaries and relevant to the total project.

The first sections of this appendix present general information pertinent to the study on Stony Run. Included are brief discussions of natural values, alternate solutions to the flood problems, and suggested items for the flood plain management program. The last section contains data and exhibits needed to make technical decisions for regulation and use of the flood plain.

DESCRIPTION OF STUDY AREA

Upstream Drainage Area

The Stony Run drainage area comprises 7.7 square miles above its mouth at the South Fork Shenandoah River (see Figure 1). The Shenandoah River is a subbasin of the Potomac River which is in the Mid-Atlantic Region as designated by the Water Resources Council. The USGS Hydrologic Unit code number in the area is 02070005. The watershed is in the Appalachian Ridges and Valleys physiographic province. Soils in the upper third of the drainage area are formed mainly in residuum from acid sandstone, shale, and phyllites on mountain slopes. Berks-Laidig-Weikert is the predominant soil series. Soils in the lower two-thirds of the watershed are formed in residuum from limestone, sandstone, and shale on uplands in limestone valleys. The predominant soil series is Frederick-Lodi-Rock Outcrop. Upland land use is about 30 percent rural residential, farmstead, and other built-up areas including a ski resort in the headwaters. The remainder includes about 34 percent woodland, 28 percent cropland, and 8 percent pasture, meadow and idle brushland.

Flood Plain

The study area includes the flood plain along 4.4 miles of Stony Run. It extends from the junction at South Fork Shenandoah River up through the community of McGaheysville to the vicinity of Hershberger Gap. Land use in the flood plain is about 45 percent pasture, hay and meadow, 21 percent cropland, 22 percent idle brushland, 7 percent woods and 5 percent miscellaneous. About 50 bridges, dwellings, farm buildings and other structures would be subject to varying amounts of damage during extreme floods.

Natural and Beneficial Flood Plain Values

The stream corridor through the flood plain has limited potential for nongame fish and wildlife habitat. Development and intensive use has virtually destroyed even this limited value except in the reaches still devoted to agriculture. Streambank stabilization and protection and establishment of vegetation cover are the primary needs to restore this potential.

FLOOD HISTORY

Flooding on Stony Run usually results from intense thunderstorm activity. Excess rainfall concentrates quickly on the steep slopes; flood stages rise rapidly and fall just as quickly. Limited flooding and damage may occur several times each year. On average, moderately severe damages are experienced at three to five year intervals. No records or recollections were noted on unusually severe floods on Stony Run. Average annual flood damages were estimated at \$10,000 to \$15,000.

FLOOD POTENTIAL

Present Conditions

Extreme floods would inundate about 127 acres of primarily agricul-tural land (see Table below). Extensive damage would be done to the land, crops, fences, farm roads, buildings and machinery. Less extensive but more critical damage would accrue to dwellings and businesses. Velocities would average about five feet per second and exceed seven feet per second in some reaches. Out-of-bank stages would range from about two to ten feet. Duration of flooding would seldom exceed six hours except during storms of intense and prolonged rainfall. Figure 1 and Figure 2 show potential flood stages at McGaheysville and at a bridge downstream.

The acres tabulated below are used primarily for pasture and other agricultural uses. Only about five percent is occupied by structure sites, but varying amounts of damage would occur to 23 dwellings, 6 trailers, 11 farm buildings, 2 commercial structures and 11 bridges.

Acres Inundated

| Type of Damage | 100-year flood | 500-year flood |
|----------------|----------------|----------------|
| Agricultural | 101 | 120 |
| Miscellaneous | 6 | |
| TOTAL | 107 | 127 |



Figure 1. Looking east across Stony Run in McGaheysville, Virginia at State Route 996 (old US Highway 33).



Figure 2. Looking north, at State Route 649 crossing of Stony Run south of McGaheysville.

Limitations on Use of Data. The flood elevations given in this report should be considered as minimum elevations. During floods, uprooted trees and other debris may collect on bridges and culverts and clog the channels. Such obstructions increase the depth and extent of flooding. Analyses were made without showing the effects of potential obstructions. Also, extremely rare events such as dam failure and climatic changes were not analyzed.

Future Conditions

The hydrologic conditions in the upstream areas are expected to improve as farmers and foresters continue to apply good management and conservation practices. This improvement is expected to reduce runoff approximately to the extent that additional development will increase runoff. Therefore, the flood hazard and damage potential is not expected to change significantly in the next 10 to 15 years.

FLOOD PLAIN MANAGEMENT

The main report includes a discussion of existing programs, current regulations, availability of flood insurance, recommendations, and related items relevant to the total study. The items discussed below relate only to Stony Run.

Floodway. The data for a "first trial" or computed floodway is filed with the basic data for Stony Run. The results indicate that hazardous conditions of depth and/or velocity prevail at current 100-year flood levels in all reaches, and that generally no additional encroachment should be allowed. The data can be used as a basis for further study of local measures, but it is suggested that no continuous or extensive floodway be considered.

Recommendations

In preparation of their comprehensive flood management program, the local sponsors should implement the following recommendations on Stony Run:

- -- Monitor future developments in the watershed to assure that regulations are followed so as not to increase the flood hazard, particularly in the drainage area above McGahevsville:
- -- Assist landowners in studies of local protection measures to reduce streambank erosion and the spread of floodwaters; and
- -- Encourage the re-establishment of natural vegetation in the flood plain to restore the fish and wildlife habitat.

Evaluation of Potential

The potential for reducing the flood hazard on Stony Run is limited by the relatively low value of damages from flooding. Yet, the damages are great enough that the "do-nothing" alternate does not warrant serious consideration.

Conversely, a brief study of contour maps indicates that flood control dams could not be economically justified. On the steep gradients, construction costs added to the costs of land rights would be excessive when related to resultant benefits.

Hydrologic conditions under current land use and management practices are generally good to excellent. An improved conservation use-and-land treatment only program would provide only limited reductions in runoff and flood stages.

These observations apply generally to all the study tributaries as do the recommendations listed in the main report. The primary opportunities have to do with prohibition of future construction or other encroachment in the flood plains; and with other regulations needed to avoid increased runoff and to minimize flood damages.

TECHNICAL DATA AND EXHIBITS

This section provides the data and exhibits needed by user agencies and individuals to make technical decisions and to comply with regulations on use of the flood plain on Stony Run.

The index map shows the area covered by the individual photomaps. Flood hazard photomaps show the area inundated by the 100 and 500-year floods. Where only one line is shown, there is no significant difference in the boundaries of the two flood areas. These photomaps should only be used to determine approximate flood elevations; they are based on semicontrolled mosaics and the boundaries shown may vary from the location on the ground.

Flood profile plates provide elevations of the 10, 50, 100 and 500-year floods at any location along the length of the streams. The elevations and discharges of the 10, 25, 50, 100 and 500-year flood at each surveyed cross section are given in Table SR-1. Sample cross sections illustrated how the flood area boundaries were located. Table SR-2 provides the description and elevation of benchmarks which are located on the photomaps.

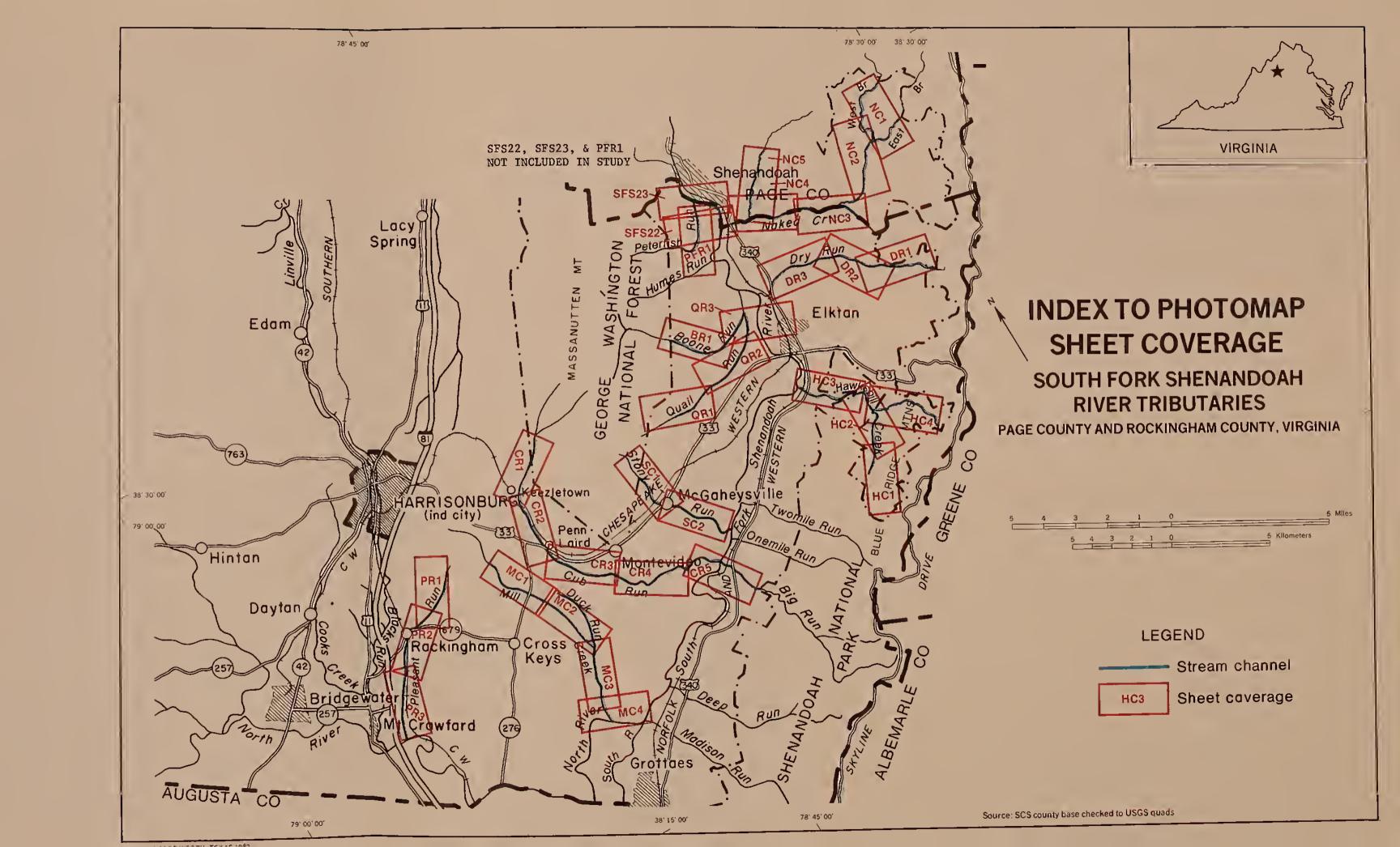
Table SR-1 can be used to locate flood elevations on the ground at surveyed cross sections.

The photomaps, flood profiles and bench mark data can be used to locate flood elevations between surveyed cross sections, as follows:

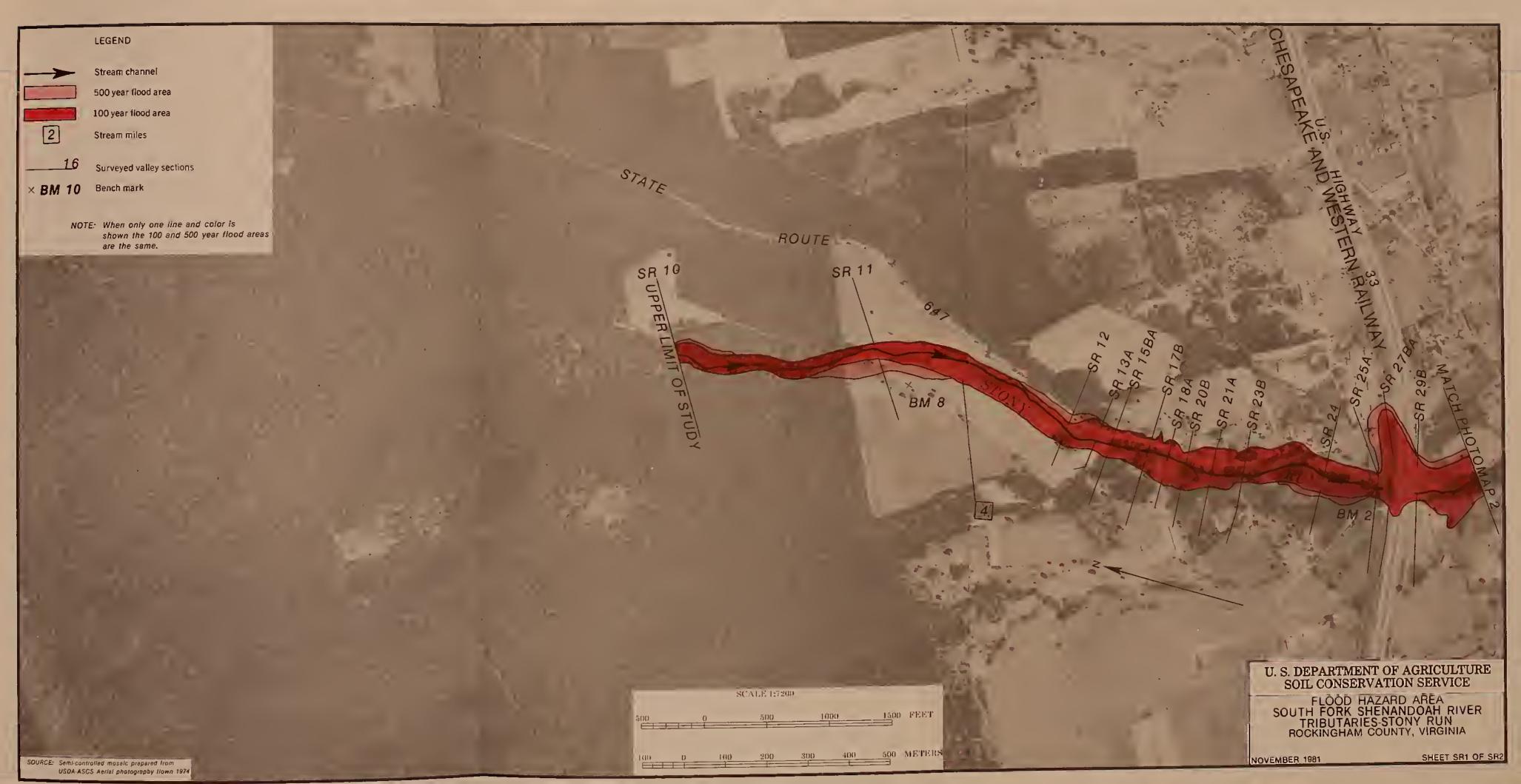
- 1. On the appropriate photomap find the point on the stream where the flood line is to be located; then scale the distance along the stream to the nearest cross section.
- 2. On the appropriate flood profile sheet, scale the distance determined in Step 1 from the cross section back to the original stream location, and read the elevation of the desired flood frequency line.
- 3. Transfer the elevation determined in Step 2 to the ground from the nearest established benchmark.

A glossary, bibliography and discussion of technical procedures are included in the main report for this study. The basic data is on file in the office of the USDA Soil Conservation Service, Richmond, Virginia 23240.

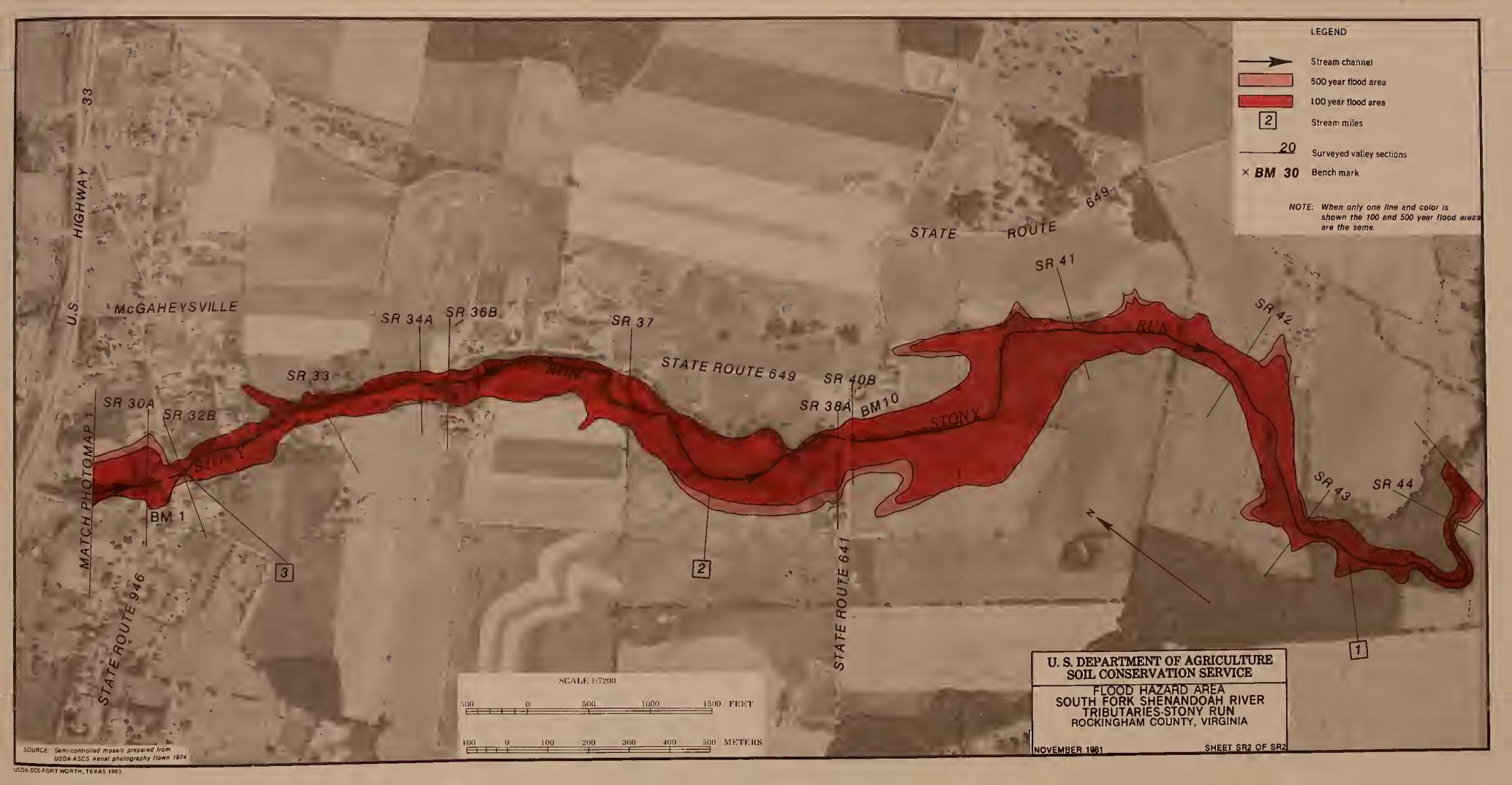




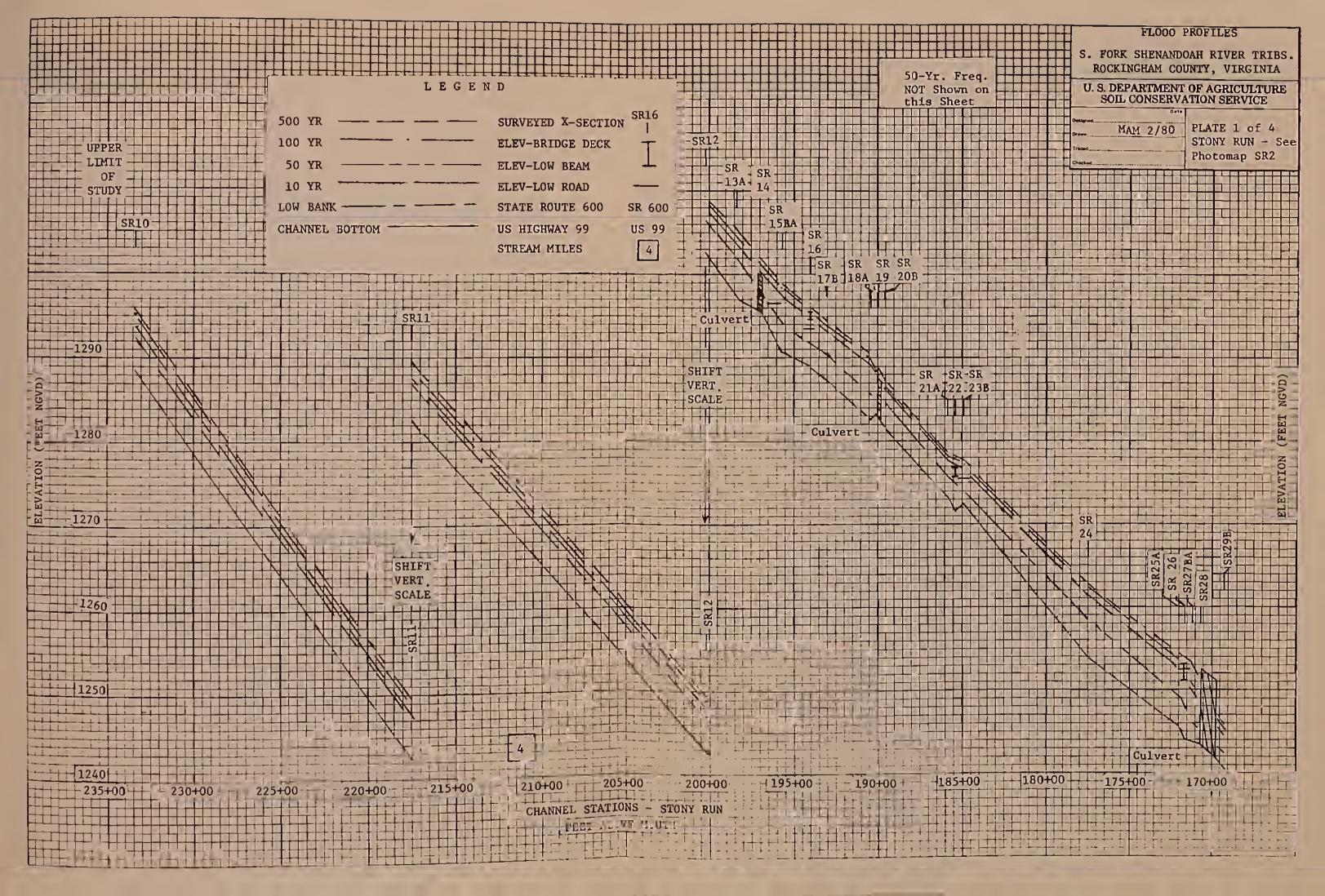


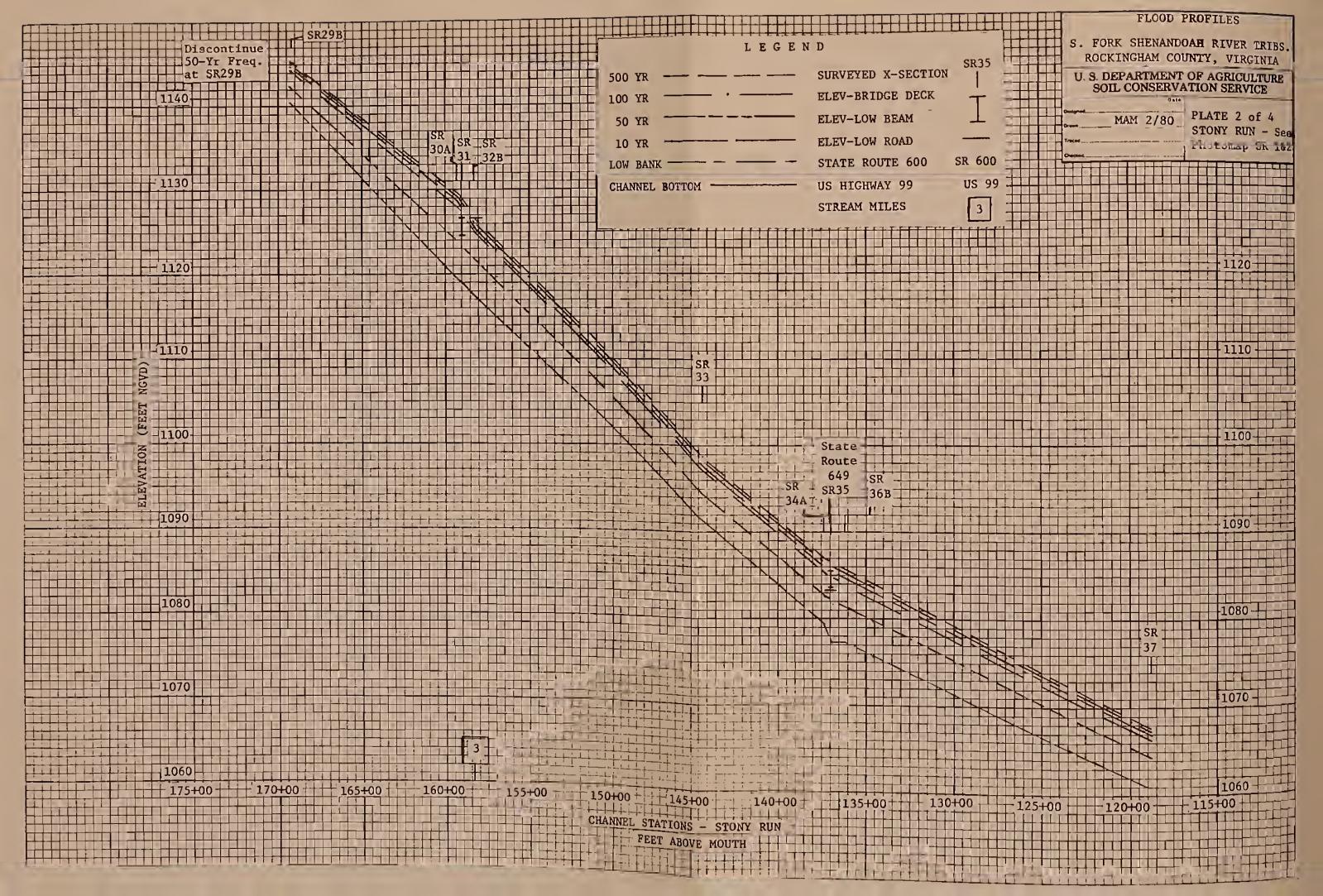


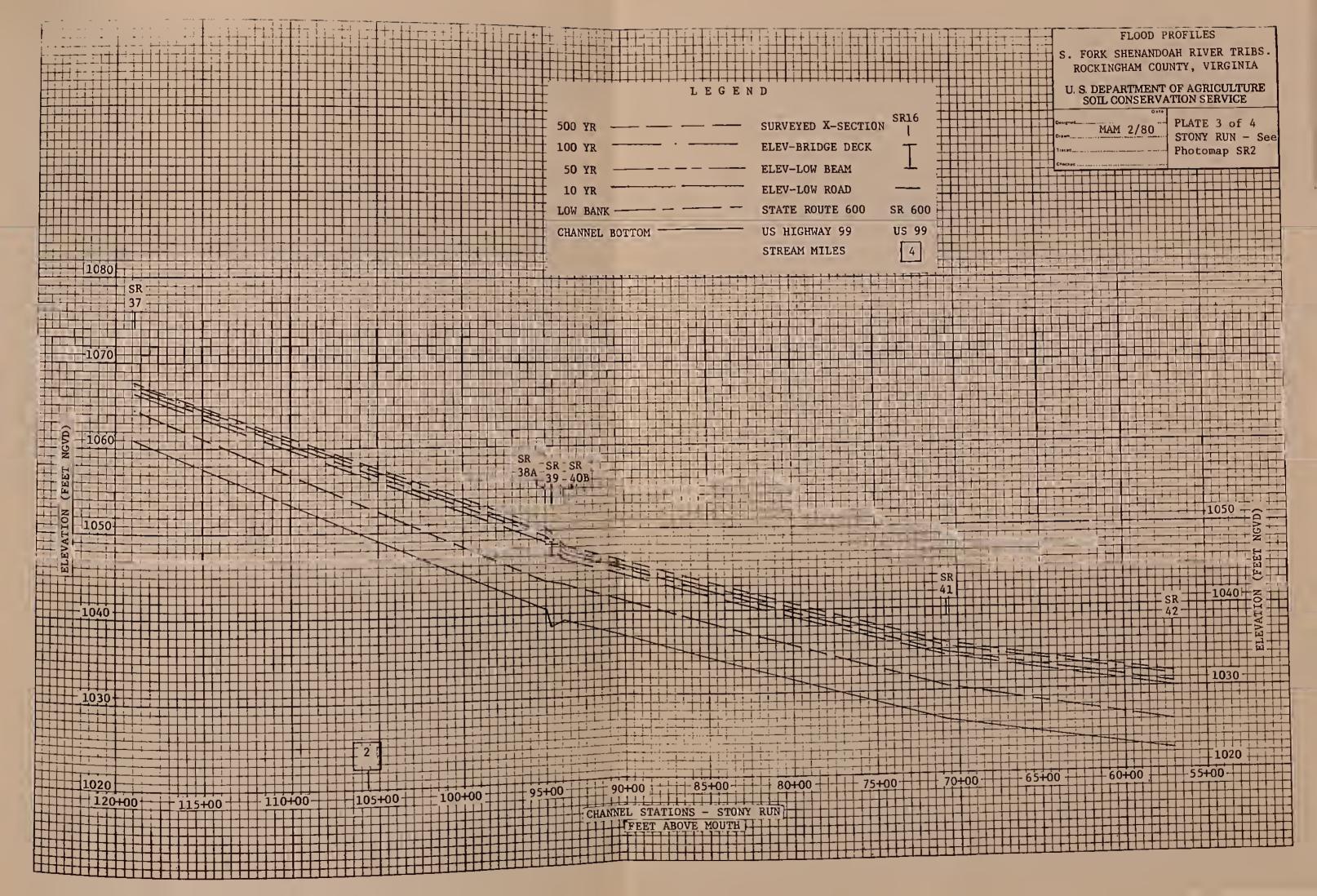


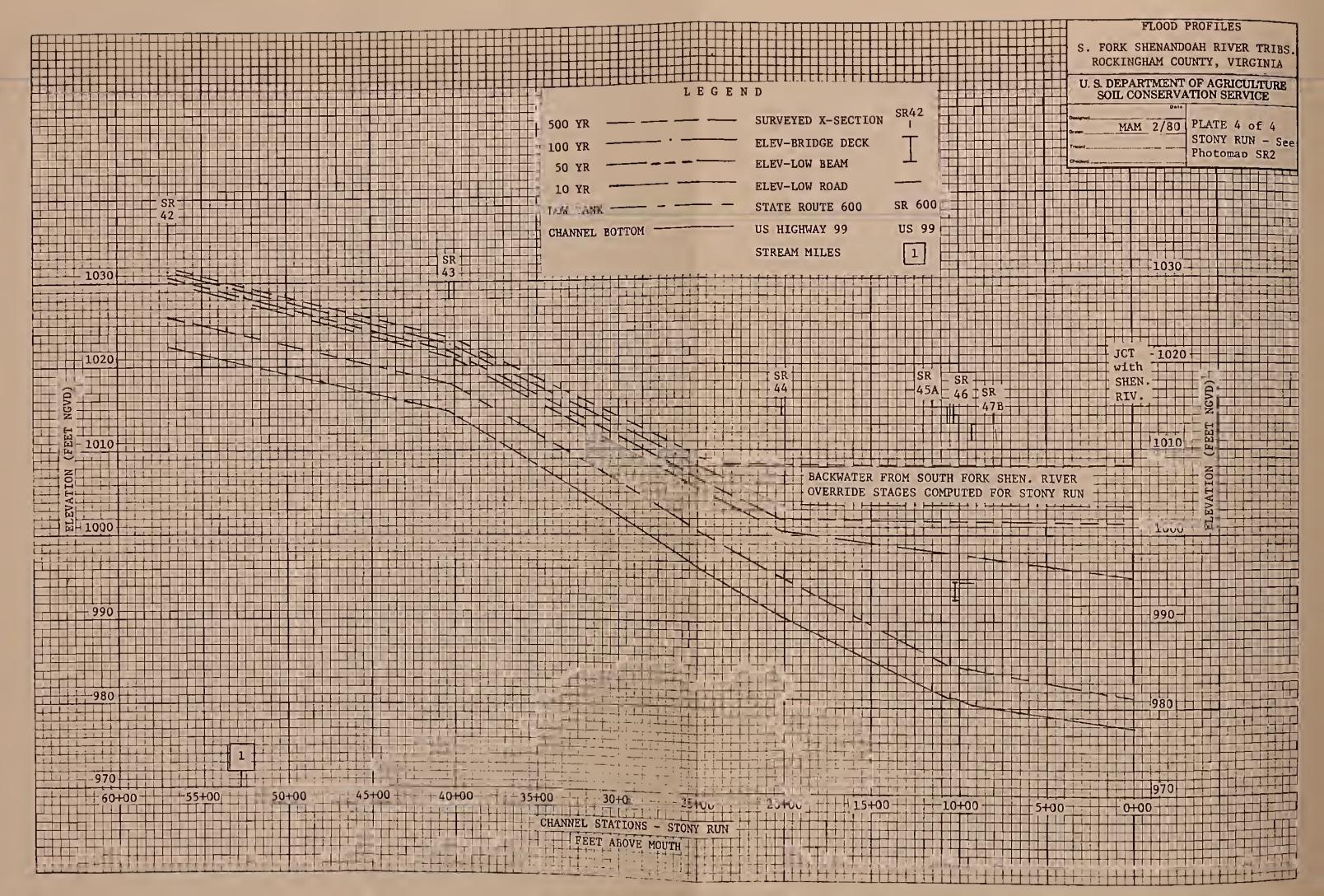












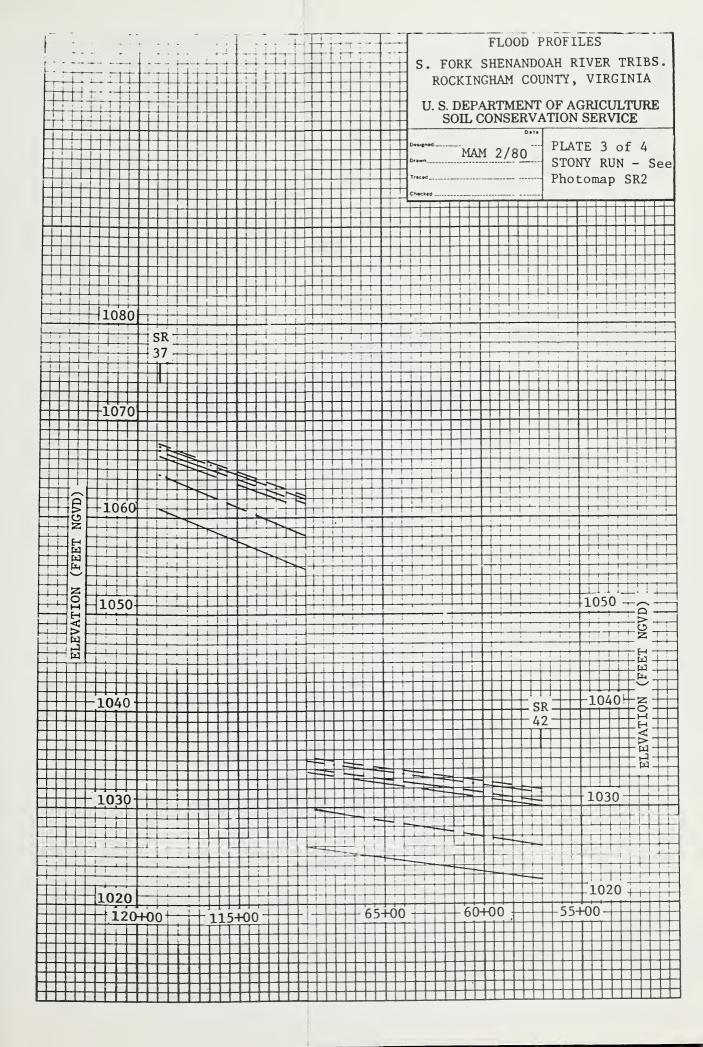


Table SR 1 Frequency-discharge-elevations, Stony Run South Fork Shenandoah River Tributaries, Rockingham County, Virginia - Continued

| 7 | Elev. | (pagu) | | 1295.7 | 1249.3 | 1209.3 | 1205.2 | | 1198.9 | | 1194.6 | 1190.2 | | 1187.0 | 1178.8 | | 1177.5 | 1162.6 | 1154.1 | | 1153.2 | | 1145.5 | 1129.5 | 26.7 | 1126.5 | 1098.9 | 1087.0 | | 1085.8 | 1067.7 | 1050.2 | | 1048.1 |
|----------|----------|---------|-----------|--------|--------|--------|--------|----------|--------|-----------|--------|--------|-----------|--------|--------|-----------|--------|--------|----------|-----------|--------|-----------|--------|--------|-----------|--------|--------|--------|----------|--------|--------|--------|-----------|--------|
| 500-vear | Disch. | | I | 2970 | 2970 | 3020 | 3030 | | 3030 | 3.5 | 3030 | 3030 | | 3030 | 3030 | 75.3 | 3030 | 3110 | 3120 | 4.5 | 3120 | 0:: | 3160 | 3720 | road 112 | 3720 | 3840 | 3860 | 1083.3 | 3860 | 3890 | 3980 | 1048.4 | 3980 |
| -vear | Elev. | (pagu) | | 1295.3 | 1248.8 | 1209.0 | 1204.7 | | 1198.5 | road 119 | 1194.2 | 1189.9 | | 1186.7 | 1178.5 | road 117 | 1177.3 | 1162.4 | 1153.7 | road 1152 | 1152.8 | road 1152 | 1145.3 | 1129.1 | 4.6, Low | 1126.0 | 1098.5 | 1086.6 | Low road | 1085.4 | 1067.5 | 1049.8 | Low road | 1047.7 |
| 100- | Disch. | (cfs) | | 2350 | 2440 | 2460 | 2480 | 6.5 | 2480 | 94.5, Low | 2480 | 2480 | 86.0 | 2480 | 2480 | 5.8, Low | 2480 | 2540 | 2540 | .0, Low | 2540 | .9, Low | 2570 | 3060 | teel 1124 | 3060 | 3180 | 3190 | 1083.6, | 3190 | 3210 | 3270 | 1047.0,] | 3270 |
| 50-vear | Elev. | (pagu) | | 1294.9 | 1248.4 | 1208.7 | 1204.4 | road 119 | 1198.2 | steel 119 | 1193.8 | 1189.7 | road 118 | 1186.5 | 1178.3 | steel 117 | 1177.0 | 1162.1 | 1153.6 | teel 1151 | 1152.6 | vert 1152 | 1145.0 | 1128.9 | .7, Low s | 1125.6 | 1098.1 | 1086.4 | ow steel | 1085.0 | 1067.3 | 1049.6 | ow steel | 1047.5 |
| 50-1 | Disch. | (cfs) | | 2020 | 2090 | 2110 | 2130 | .0, Low | 2130 | 2, Low | 2130 | 2130 | .7, Low | 2130 | 2130 | 8, Low | 2130 | 2160 | 2160 | , Low s | 2160 | ver cul | 2200 | 2620 | ck 1126 | 2620 | 2740 | 2750 | 85.5, L | 2750 | 2790 | 2800 | 48.5, L | 2800 |
| Par | Elev. | (pagu) | | 1294.6 | 1248.0 | 1208.5 | 1204.0 | ert 1200 | 1197.9 | ck 1195. | 1193.5 | 1189.5 | ert 1187 | 1186.4 | 1178.0 | ck 1176. | 1176.9 | 1161.8 | 1153.4 | k 1153.0 | 1152.4 | t, Hwy 0 | 1144.9 | 1128.6 | ridge de | 1125.4 | 1097.8 | 1086.0 | deck 10 | 1084.6 | 1067.0 | 1049.4 | deck 10 | 1047.3 |
| 25-vear | Disch. | (cfs) | | 1720 | 1790 | 1810 | 1820 | ver culv | 1820 | Sridge de | 1820 | 1820 | Over culv | 1820 | 1820 | ridge de | 1820 | 1860 | 1860 | idge dec | 1860 | 3-Culver | 1890 | 2240 | ay 33, B | 2240 | 2340 | 2350 | , Bridge | 2350 | 2380 | 2400 | , Bridge | 2400 |
| 0-vear | Elev. | (pagu) | | 1293.9 | 1247.6 | 1208.0 | 1203.4 | road, 0 | 4 | road, B | a | 1189.1 | road, 0 | 1186.1 | 1177.7 | road, B | 1176.5 | 1161.3 | 1152.4 | lway, Br | 1151.0 | lighway 3 | 1144.7 | 1128.1 | S. Highwa | 1125.0 | 1097.3 | 1085.5 | oute 649 | 1084.0 | 1066.6 | 1048.6 | Route 641 | 1046.7 |
| 10-0 | | | | 1320 | 1370 | 1380 | 1380 | Private | 1390 | Private | 1390 | 1390 | Private | 1390 | 1390 | Private | 1390 | 1420 | 1420 | N&W Rai | 1420 | U.S.H | 1440 | 1710 | Old U. | 1710 | 1790 | 1800 | State R | 1810 | 1820 | 1830 | State R | 1830 |
| | DA | (sq mi) | limit of | 3.76 | 3.90 | 3.97 | 3.98 | 3.98 | 4.00 | 4.00 | 4.00 | 4.03 | 4.03 | 4.04 | 4.07 | 4.07 | 4.07 | 4.12 | 4.15 | 4.15 | 4.15 | 4.16 | 4.20 | 5.45 | 5.45 | 5.46 | 5.91 | 5.97 | 5.97 | 5.98 | 6.14 | 6.30 | 6.30 | 6.31 |
| Profile | Plate | No. | - Upper | - | 1 | 1 | 1 | | - | 1 | 1 | - | 1 | 1 | 1 | 1 | 1 | - | ~ | 1 | | 1 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2&3 | က | က | က |
| | Photomap | No. | Stony Run | SR1 | SR1 | SR1 | SR1 | SR1 | SR1 | SR1 | SR1 | SR1 | SR1 | SR1 | SR1 | SR1 | SR1 | SR1 | SR1 | SR1 | SR1 | SR1 | SR1&2 | SR2 | SR2 | SR2 | SR2 | SR2 | SR2 | SR2 | SR2 | SR2 | SR2 | SR2 |
| | 114 | X-Sec. | נטן | SR10 | SR11 | SR12 | SR13A | SR14R | SR15BA | SR16R | SR17B | SR18A | SR19R | SR20B | SR21A | SR22R | SR23B | SR24 | SR25A | SR26R | SR27BA | SR28R | SR29B | SR30A | SR31R | SR32B | SR33 | SR34A | SR35R | SR36B | SR37 | SR38A | SR39R | SR40B |

South Fork Shenandoah River Tributaries, Rockingham County, Virginia - Continued Table SR 1 Frequency-discharge-elevations, Stony Run

| year | Elev. | (pagu) | 1035.9 | 4300 1031.9 | 1023.4^{2} | | | | | | 1009.0** |
|---------|----------|---------|--------|-------------|--------------|---------|-------|---|-------|---------|----------|
| 500- | Disch. | (cfs) | 4150 | 4300 | 4390 | مد | | 9.466 | | | j; |
| -year | Elev. | (pagu) | 1035.3 | 3550 1031.3 | 1022.6 | 1003.5 | | w road | | | 1004.0** |
| 100 | Disch. | (cfs) | 3410 | 3550 | 3600 | . 3800 | | 92.4, L | | | ÷ |
| /ear | Elev. | (pagu) | 1035.0 | 3010 1030.9 | 1022.2 | 1002.6% | | Route 650, Bridge deck 994.6, Low steel 992.4, Low road 994.6 | | | 1001.5** |
| 50-3 | Disch. | (cfs) | 2940 | 3010 | 3090 | 3210 | | 14.6, Lov | | | de |
| rear | Elev. | (pagu) | 1034.8 | 2650 1030.6 | 1021.9 | 1001.9 | | deck 99 | | | 999.7** |
| 25-3 | Disch. | (cfs) | 2550 | 2650 | 2700 | 2790 | | , Bridge | | | |
| year | Elev. | (pagu) | 1034.4 | 1030.0 | 1021.1 | 1000.5* | | oute 650 | | | 995.1** |
| 10-y | Disch. | (cfs) | 1930 | 1990 | 2000 | 2120 | | State R | | | |
| | DA | (sq mi) | 6.73 | 7.02 | 7.19 | 7.64 | 7.69 | 7.69 | 7.70 | | 7.70 |
| Profile | Plate | No. | 3 | 3&4 | 4 | 7 | 7 | 7 | 4 | | 7 |
| | Photomap | No. | SR2 | SR2 | SR2 | SR2 | | | | th | en |
| | | X-Sec | SR41 | SR42 | SR43 | SR44 | SR45A | SR46R | SR47B | Jct Sou | Fk Sh |

For 10, 25, 50 and 100-year stages below X-Sec. SR 44, and for 500-year stages below X-Sec. SR 43, see Flood Hazard Study for South Fork Shenandoah River and North River. Backwater from the main stem of South Fork overrides the stages on Stony Run in this reach.

** Stages on South Fork Shenandoah at mouth of Stony Run.

Table SR-2 Benchmark descriptions, Stony Run, Rockingham County, Virginia - 1979

| B.M. No. | Photo Sheet No. | Description, location and elevation |
|-------------|-----------------------|--|
| 8 | SR-1 | SCS TBM - near X-sec. SR-11, 150 feet downstream, a metal disk is located in the base of power pole #23, 250 feet southwest of dwelling and 200 feet east of barn. Elevation 1244.13 |
| 2 | SR-1 | SCS TBM - A square is chiseled on the upstream (northwest) head wall of concrete culvert of the westbound lane of U.S. Hwy 33 over Stony Run at McGaheysville, Virginia. Elevation 1150.88 |
| 1 | SR-2 | SCS TBM - a square is chiseled on the upstream (southeast) abutment of bridge over Stony Run on State Route 996 in McGaheysville, Virginia. Elevation 1127.35 |
| 10 | SR-2 | SCS TBM - A square is chiseled on top of the downstream (southeast) abutment of bridge No. 6039 over Stony Run on State Route 641, approximately 400 feet west of Jct. with State Route 641 & 649. Elevation 1048.70 |

Note: Elevation in feet above National Geodetic Vertical Datum of 1929.



